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Presented at the FIG Working Head Geodetic Leveling in the Modernized National Spatial Reference System

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Differential leveling



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Topography

Differential leveling

Differential leveling

- Very accurate differential heights. Precision < 1 mm.
- Time consuming, expensive.
- For absolute positioning, starting heights must be known.

Current method:

- Start from passive bench marks with heights published in NGS's vertical datum, NAVD88.
- Establish heights on new marks via leveling.



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Leveling today: assumptions

- Published marks are available near the project area.
 - Historic level lines must have been previously run.
- Mark still exists and is recoverable.
 - Marks are often lost or destroyed without being replaced.
- Mark has not moved.
 - Stability of setting, damage, subsidence, uplift.
- Published coordinates are accurate & consistent.
 - Coordinates may be "stale".

Leveling in the modernized NSRS

- Starting heights need not come from historic level lines.
- Most reliable source will be from new GNSS occupations of *primary control marks* included in the level survey.
 - Static, Real-Time Kinematic, Network RTK.
 - Obtain orthometric heights from GNSS ellipsoid height and high-resolution geoid model.

GNSS-derived orthometric heights



Primary control marks

- 3 marks minimum.
 - Provide access to NAPGD2022 orthometric heights.
- 30 km max distance.
- GNSS occupations to obtain coordinates at an epoch representative of the survey (e.g., the midpoint date).
 - 2 occupations minimum at the beginning of leveling.
 - 2 occupations minimum at the end of leveling.
 - For projects over 6 months, 2 occupations near the middle of leveling.

Additional marks for leveling

- Leveling-only secondary marks are set as needed for the project.
- Nominal spacing of 1-3 km for level lines.
- Double-run leveling (forward/backward) connects the primary marks to the other marks.

GNSS and leveling

GNSS-derived orthometric heights



Double-run leveling



Credit: Dan Gillins

Establishing heights

At the primary control marks:

- GNSS occupations are processed into NATRF2022 coordinates at the survey epoch.
- Coordinates combined with GEOID2022 to produce orthometric heights at the survey epoch.
- Orthometric heights used as vertical control for adjustment of the leveling data.

Establishing heights

- Uncertainties will be propagated through the processing sequence. Sources include:
 - σ_h from GNSS observations (cm level).
 - σ_N from GEOID2022 model (cm level).
 - $\sigma_{\Delta H}$ from leveling observations (mm level).
- In the adjustment, the controlling orthometric heights will float according to their uncertainties
 - Variance component model with stochastic constraints.

Establishing heights

- Per the weighting scheme...
 - Absolute accuracy is determined by the GNSS (network accuracy).
 - Relative precision of the leveling is maintained (local accuracy).
- Corrections are applied to leveling for systematic effects.
 - Astronomic, orthometric, gravity, atmosphere, instrument calibration.
- End result: adjusted orthometric heights for all points in the project at the survey epoch.

GNSS and leveling adjustment



GNSS and leveling adjustment





Conclusions

In the modernized NSRS:

- Leveling will remain the premier technique for obtaining accurate differential heights.
- Users will longer have to rely on historic passive control with coordinates that may be unsuitable.
- Users can establish new marks and starting coordinates tied to NAPGD2022 via GNSS wherever they like.

Conclusions

In the modernized NSRS:

- High-precision leveling can extend vertical control throughout the project area.
- NGS tools will process and adjust the data, providing orthometric heights, along with estimates of uncertainty.

Under development

- Software and tools for data processing.
- Integrating functionality into the OPUS software suite.
- Making adjacent projects at different epochs consistent.
- Updating formal documentation on leveling methodologies and GNSS control.

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Thank you.