



A template for the development of a modern national reference frame

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& Quickclose



Workshop presentation overview

- What is the purpose of national geodetic reference frame?
- geodetic Infrastructure
- geodetic observations
- data processing and adjustment
- deformation and transformation modelling
- products for users and GIS

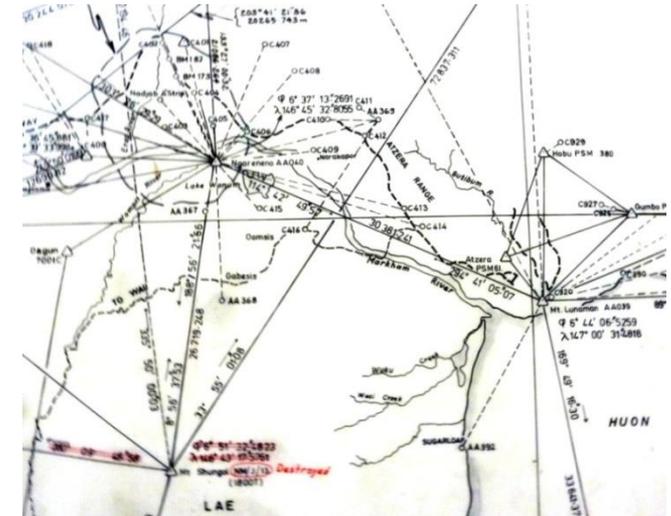


What is the purpose of national geodetic reference frame?

- Positioning and the basis for spatial data and GIS
- Cadastral (including customary land and resource lease) surveys
- Engineering surveys (roads, ports, construction, mining, oil & gas)
- Topographic Mapping & DEM (LiDar ground control and imagery control)
- Asset Mapping (e.g. GIS surveys, general features, villages, street map, TLS)
- Hazard & environmental monitoring (volcanoes, landslides, subsidence)
- Plate tectonics, seismic deformation
- Sea level change (e.g. monitoring elevation and stability of Tide Gauges)
- Contribution to global and regional geodesy (e.g. GGOS, IGS, APREF)

Monumentation – Evaluate existing infrastructure

- Identify existing primary control stations and levelled benchmarks from earlier survey networks (e.g. trig stations)
- (assess for accessibility, stability, GNSS (sky visibility), utility and proximity to development, cadastral connections)
- The purpose? To estimate transformation parameters and distortion grids from earlier geodetic datums
- traceability of geodetic frames and datums over time



AGD66 trilateration network, Morobe Province, Papua New Guinea



Monumentation – Establish CORS

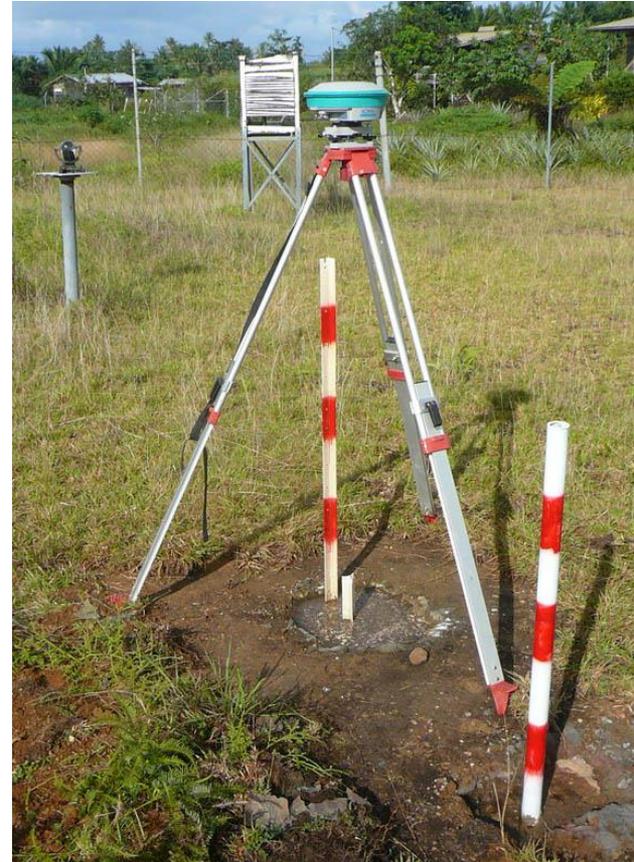
- continuously operating GNSS stations) in main towns and development areas to support RTK/NRTK and local static GNSS surveys.
- (Consider RTK and static range limitations, mobile network coverage for NTRIP, power supply and UPS backup)
- Tectonic monitoring and/or fit for purpose



COCO
Cocos Islands
IGS/ ARGN/
APREF pillar
Indian Ocean
Australia

Monumentation – Augment existing infrastructure

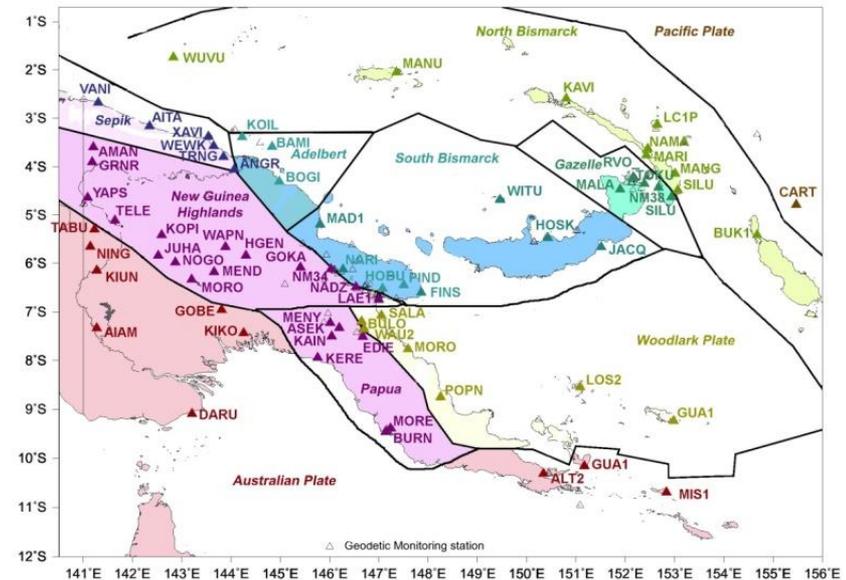
- Construct new passive geodetic stations at useful places like airports, port facilities (tide gauges), government offices, schools, playing fields, meteorological stations, resource sector camps (secure locations with no land ownership issues and good sky visibility)



Kiunga,
2nd order
station,
Western
Province,
Papua
New
Guinea

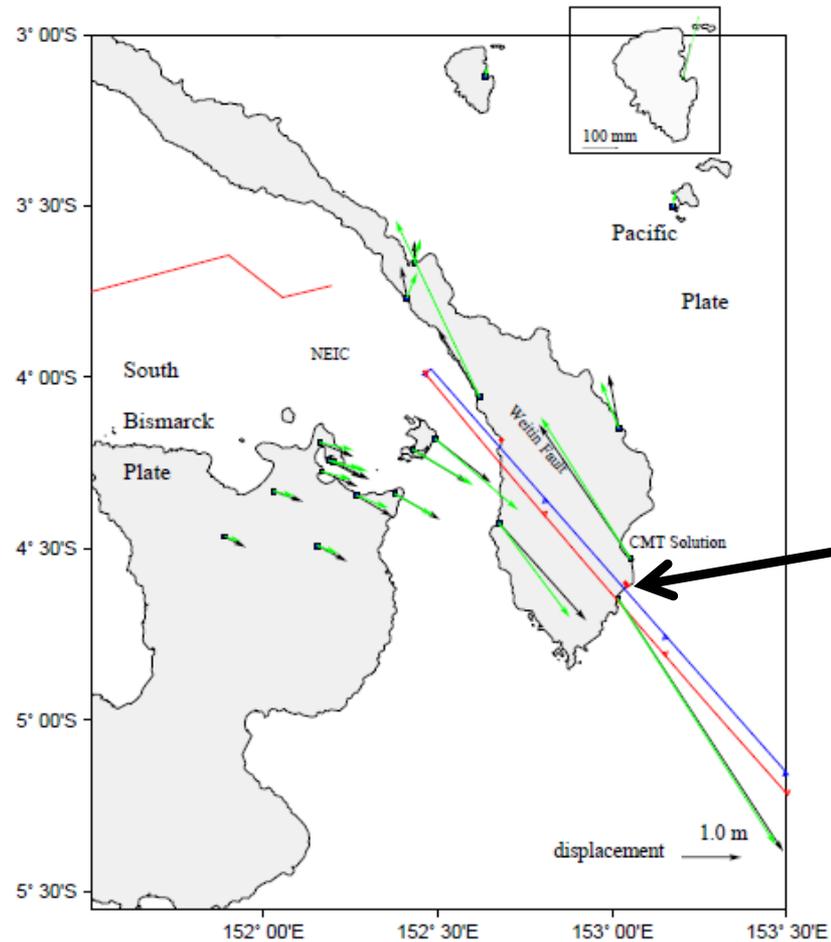
Monumentation – Tectonic & Sea Level monitoring

- Dense network of geotechnically stable geodetic monuments on either side of plate boundary or active fault zone.
- Consider optimum geometry for modelling and inversion of fault locking parameters.
- Regular network of stations within rigid portion of plate to enable inversion of plate model Euler pole.
- Siting of monitoring stations around each tectonic plate and boundary zone
- Tide Gauges well spaced around coastline away from river mouths.



Direct measurement of seismic deformation

Mw8.0 Weitin Fault earthquake,
 New Ireland, Papua New Guinea
 16th November 2000 (Tregoning, ANU)





Other geodetic networks – hazard monitoring

- Volcano monitoring networks
- Subsidence zones
(e.g. Above underground mining operations, coal-seam gas extraction, groundwater and aquifer abstraction)
- Landslide monitoring
- Localised deformation monitoring

Co-location with other geodetic sensors

**DORIS Beacon
(IDS Network)**

**Satellite Laser
Ranging?**

VLBI ?

Co-location has
very significant
benefits for
global geodesy
and ITRF



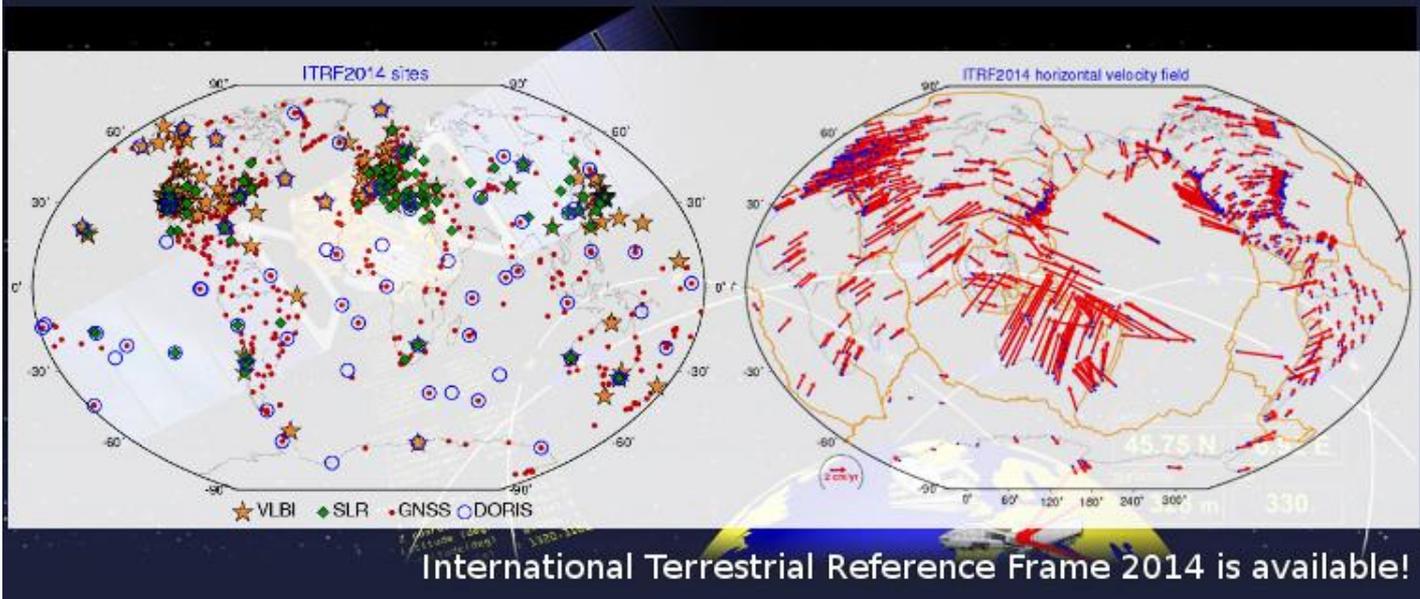
Port Moresby DORIS and APREF CORS
Papua New Guinea

**GNSS Antenna
APREF GNSS Network**

**Tie and stability
check RM
(preferably should be
instrument pillar)**

Co-location with
gravity stations

Contribution to global and regional networks



ITRF (including IGS)

APREF

(regional densification of ITRF)



Choice of passive geodetic monument construction

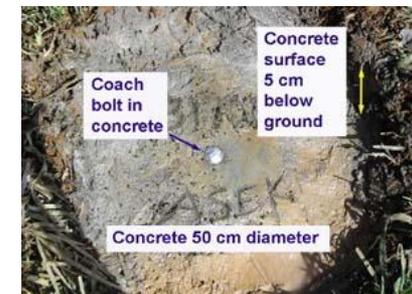
considerations:

- Cost & availability of materials
- longevity and stability of monument
- Risk of damage or vandalism (e.g. theft of brass plaques!)
- Deep footings and reinforcement if possible

Brass Plaque



Stainless steel bolt



Star Picket



Galvanised Iron Pipe



Geodetic pillars

considerations:

- Ideal for mining and CORS tie monitoring (already centred for total stations and GNSS antennas)
- Easily located
- Requires especially deep and robust footings and reinforcement

Lihir
Mine
Pillar
New
Ireland
PNG



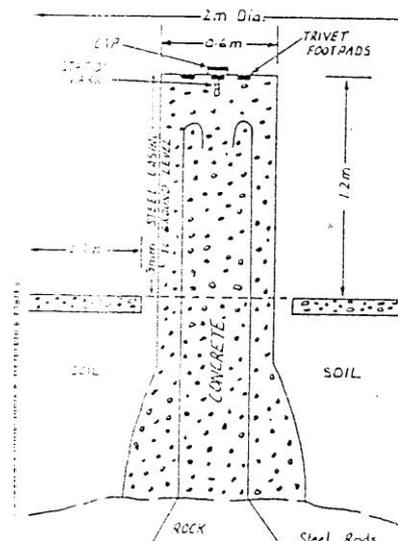
Kiunga
GPS base
Western
Province
PNG



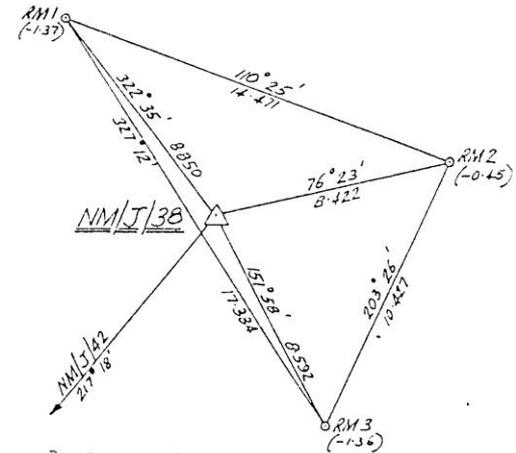
Choice of monument siting

- Sky view for GNSS observations (Proximity of trees, vegetation growth (including future growth), structures, proposed structures)
- Utility – e.g. Is it within range of working area for reliable L1 fixed solution? Intervisibility with other stations for total station use
- Risk of destruction – located away from possible earthworks or construction, vehicles.
- Stability of site – On contiguous bedrock – not floaters!
- Avoid clay or deep soils, slopes, edges – requires very deep footings.

Stability of stations – some pitfalls!



CRUSTAL MOVEMENT SURVEY
 BRIGHN PILLAR
 First Identification.



Bearings are true.
 Distances in metres.

Corrected for transcription errors: *S. Wood* Date: 11-12-75

Tinbal – Crustal Motion Pillar
 – New Ireland, PNG



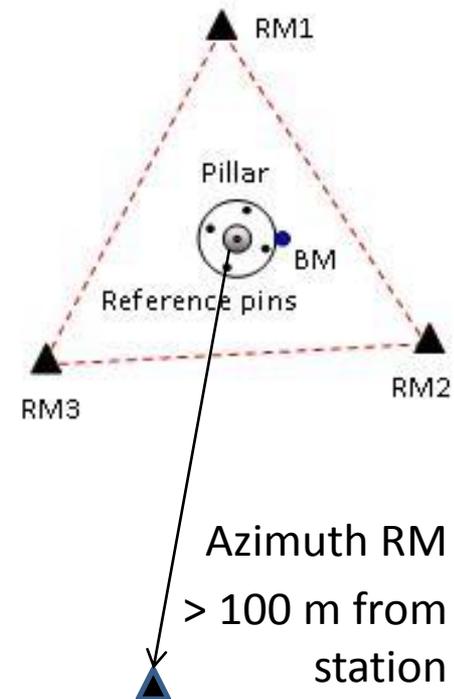
CORS monuments – good enough for Tier 1?

Roof or tower antenna mount limitations:

- Unstable structure?
- Strong winds (e.g. cyclones) can induce wind shear deformation
- Thermal expansion of structure (e.g. steel tower)
- **Best construction is a low concrete pillar** with very deep footings and reinforcement - tied to bedrock. Requires long curing time.
- Consider sky visibility and multipath (remove young trees nearby)
- Tier 3 “Fit For Purpose” CORS for lower precision usage / infill

Stability monitoring of primary control / CORS

- local RM network, low pillars, duplication (redundancy) at common sites, stability of tide gauges.
- Redundancy RMs at > 50 m to monitor site stability
- Azimuth RMs to support terrestrial surveys (e.g. cadastral and construction)
- Stable gravity station in local network





Reference Marks and Witness Posts

- RMs especially important to verify stability of primary mark (and recovery of main mark if disturbed or vandalised).
Constructed to similar standard to main mark (e.g. iron pin in concrete)
- Best located within 5 m of main mark and concealed slightly below ground level. 3 marks in a triangle around mark.
- Witness post ideally within 50 cm of station. e.g. star picket or galvanised pipe set in concrete. Also consider windsocks at airports (> 5 m away) , rugby goal posts (beyond dead ball line to avoid broken ankles), basketball posts.

Tide Gauge – siting and monitoring network



Tide Gauges sited away from river mouths – areas of strong wave action or currents
Considerations: Subsidence and disturbance to wharf
Slipping of tide gauge zero mark over time – damage
Important to have nearby BM on bed-rock away from wharf



Observations – Choice of equipment

- **GNSS sensors:**
- L1 only – limited to 10-15 km for fixed solution or so (cheaper)
- L1/L2, L1/L5 – anywhere on the Earth (more expensive)
- GPS only, GPS + Glonass, GPS + Galileo
- Beidou(Compass), QZSS, IRNSS capability
- Carrier-phase processing not yet fully interoperable with some GNSS services (e.g. GPS and FDMA Glonass)
(so multi-GNSS of limited value for static GNSS)
e.g. GPS only fixed solution + Glonass only float solution

Choice of equipment considerations

- Does the equipment have a good warranty and reputation?
- Use a local supplier for warranty and ex- warranty support & repairs – even if it costs more.
(customs hassles and 2-way air freight is also expensive!)
- Is the equipment robust (water proof) for extreme conditions?
- Do other organisations nearby have similar equipment?
- Antenna calibration (GEO++, NGS ANTICAL or individual antenna?)
- Remote area extras: external batteries and cables, spares
- Ongoing equipment maintenance budget.



GNSS Observations for fiducial (zero order) network

- Ideally Tier 1 CORS monument for continuous measurement.
Or campaign style observations, if there are budget constraints.
- For fiducial network recommend multi-day 24 hr observations (e.g. 7 day observations to mitigate unmodelled ocean-tide loading effects which primarily affect vertical precision)
- Repeat observations every six months for two to four (or more) years in order to model station time series in ITRF and average out seasonal signals e.g. draconitics, unmodelled hydrological and atmospheric pressure loading
- *5 hours of dual-frequency carrier-phase GPS observations can provide 15 mm precision in ITRF (30 mm for ellipsoidal height)*



GNSS static observations for lower order networks

- GNSS base station running over CORS or fiducial station
- GNSS rover stations running at stations within radius of 30 km in order to optimise observation time and minimise tropospheric modelling errors and other common mode errors.
- Observation time 15 minutes to 2 hours depending upon baseline length, Satellite geometry (GDOP), availability and observing conditions (e.g. longer obs required if station near trees or buildings)
- 3+ receivers running concurrently provides redundancy. Unchecked baseline radiations are inadvisable.



GNSS observations on older datum stations

- Important for estimating transformation between old and new datums to enable legacy spatial data (e.g. Topographic, engineering and cadastral plans) to be transformed accurately to a new datum – **maintaining traceability for spatial data.**
- Observe dense network in urban areas for high precision estimation (and evaluation) of parameters – typically a block shift, affine transformation, multi-parameter transformation or distortion grid.
- Locate bench marks (with local height datum) in order to estimate offset model between geoid model and local height datum surface (single offset, transformation plane or grid).



Other geodetic measurements

- Total station measurements for site ties, RM surveys, observations to geodetic control (especially legacy control) under trees.
- EDM calibration baselines with stable instrument pillars
- **Important considerations:** using realistic atmospheric corrections in EDM equipment (e.g. atmospheric pressure and temperature – especially important for long EDM measurements and at higher elevations). 90 ppm correction typical at 3000 metre elevation.
- Verify prism constants
- Levelling ties at tide gauges to monitor TG stability.
- Sea level measurements at tide gauges



GNSS Data processing and adjustment - Choice of software

- Can the software do dual-frequency carrier-phase processing?
- Can the software do network adjustment with weighting options?
- Does software support projected coordinates, geoid models?
- Can software utilise IGS precise orbits (e.g. Sp3)?
- Are different tropospheric and ocean-tide loading models selectable?
- Multiple licences for field use – support agreement indefinite?
- **Bernese software (GNSS)** – widely used and supported – expensive
\$\$\$\$ - Linux and Windows OS
- **GAMIT/GLOBK (GPS)** – less well used, ideal for batch processing– AND
free for non-commercial applications – Unix/Linux OS.
- **Trimble Business Centre, Leica Geo O, Topcon Tools** – user friendly - \$\$
- **RTKLIB** – open source

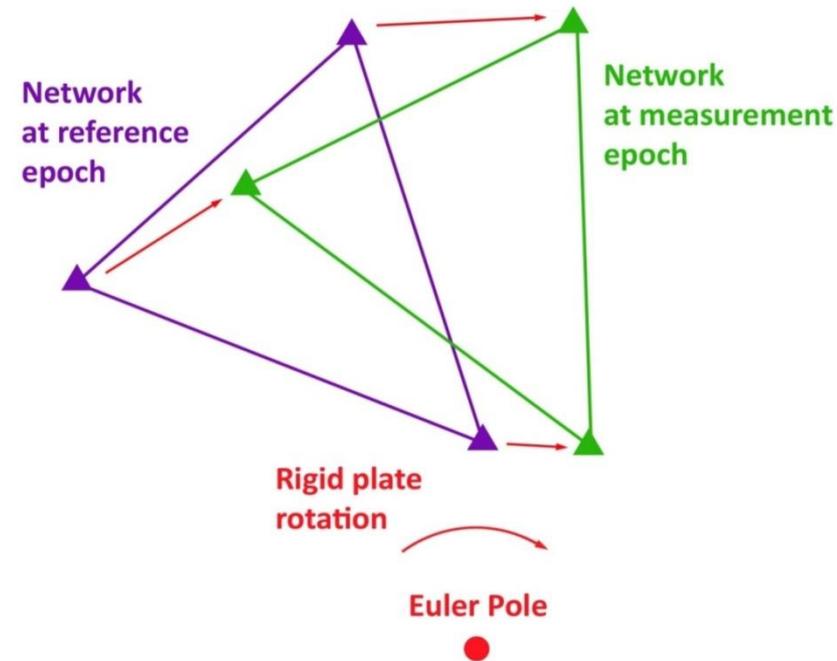


Data processing and adjustment – Online services AUSPOS, OPUS, TRIMBLE-RTX, NRCAN-PPP, POSITIONZ-PP etc.

- Relatively painless method of data processing (for RINEX)
- AusPOS and PositionZ-PP (Bernese), Opus(PAGES),
- No cost involved
- ITRF2008 (or soon ITRF2014) and local datum coordinates
- EGM2008 and local geoid model & uncertainty
- 5 hours data -> 15 mm Hor. & 30 mm Vert.
- Wait 2+ days for IGS Rapid orbit
- ITRF Agreement between different services typically 3-5 mm for 24 hr observations

Choice of reference frame for GNSS data analysis

- ITRF at mean epoch of measurement is optimal
- Overcomes adverse effects of unmodelled localised deformation and plate rotation between reference epoch and epoch of measurement
- Transform solution to local frame/datum/fixed epoch of ITRF after adjustment.



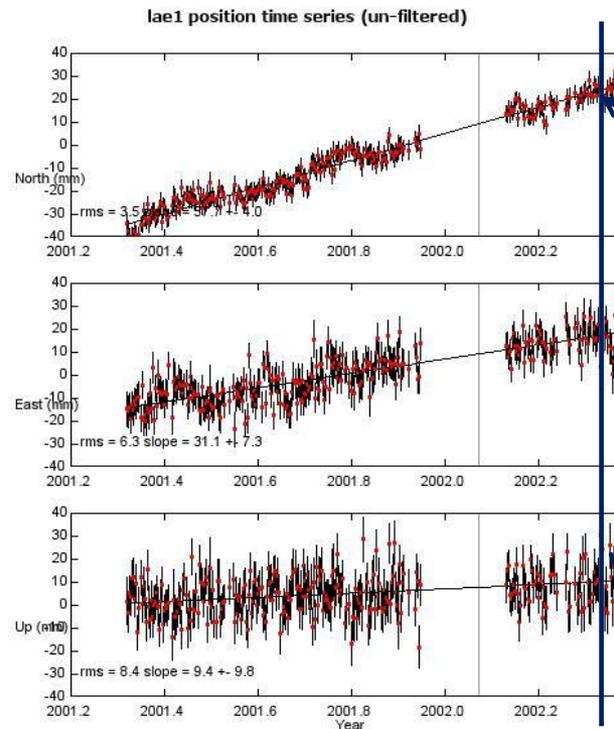
Model station time-series in ITRF to estimate site velocity & reference epoch

Recommended approach
for local datum reference
epoch:

Choose epoch near end of
timeseries.

Consider reference
epochs of adjoining
jurisdictions

Spectral analysis of time
series to model noise



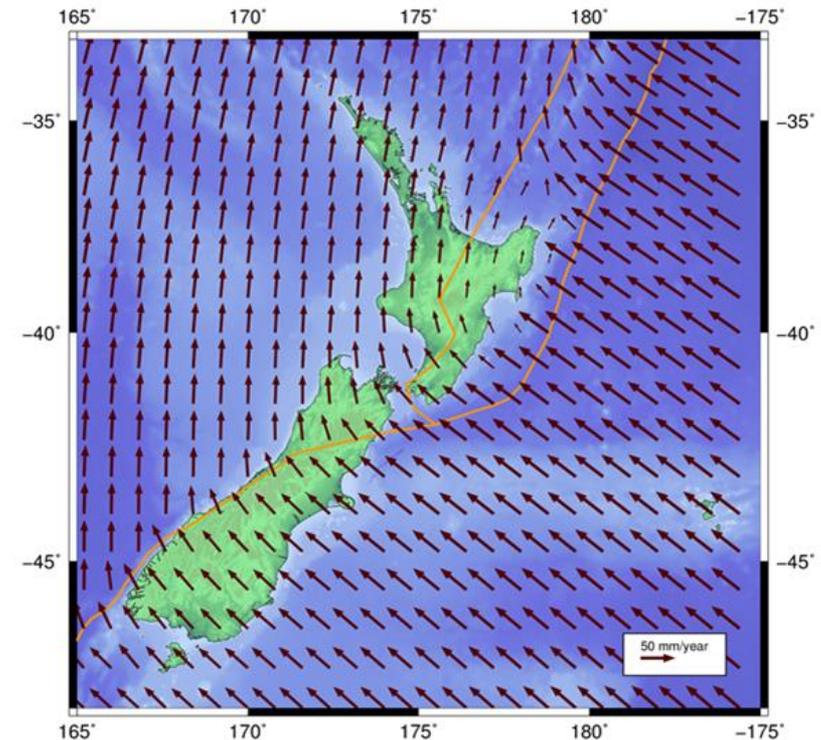
Select reference
epoch for local
frame (datum)
determination

e.g. 2003.0

Develop velocity (secular deformation) model

Enables ITRF coordinates at epoch to be propagated to another epoch (e.g. local datum reference epoch) to model out underlying secular plate motion.

Alternatively a stable plate model by inversion of Euler Pole from site velocities, 14 parameter, 6 parameter or block shift rate can be derived (e.g. for smaller island states located away from plate boundaries)

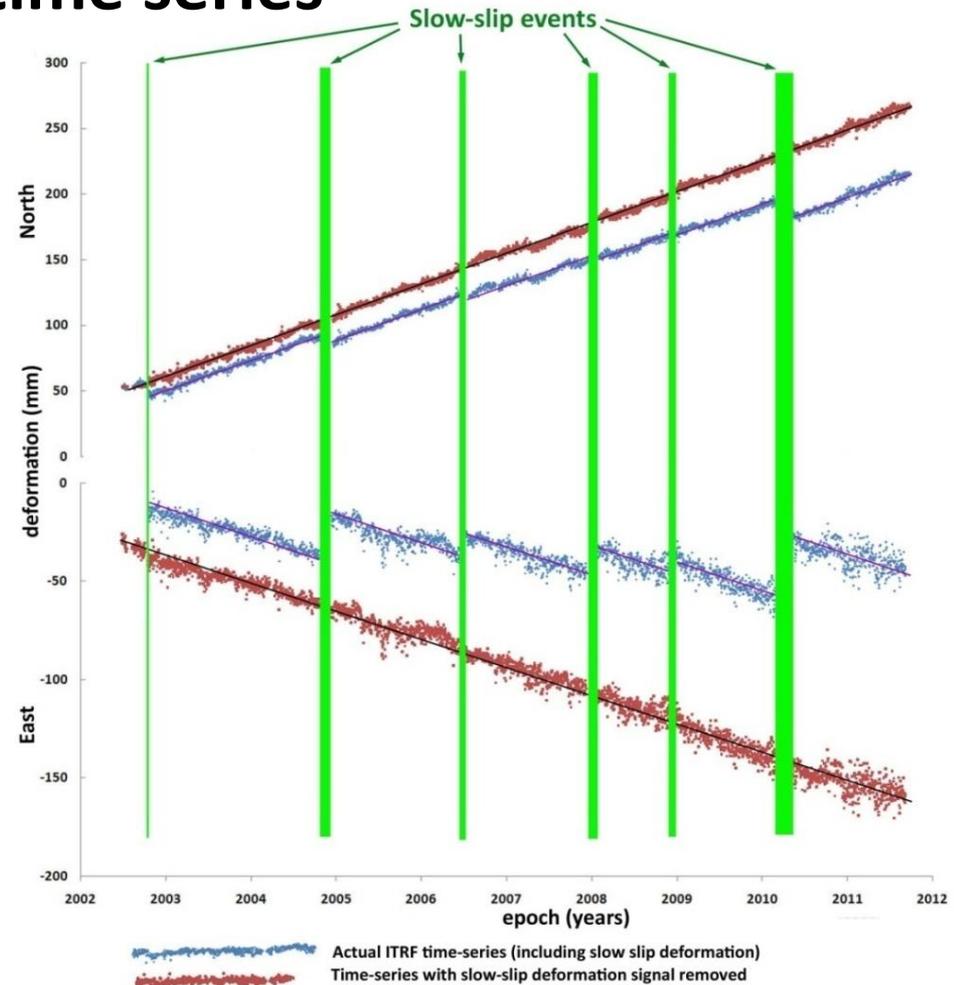


Estimate seismic offsets in time series

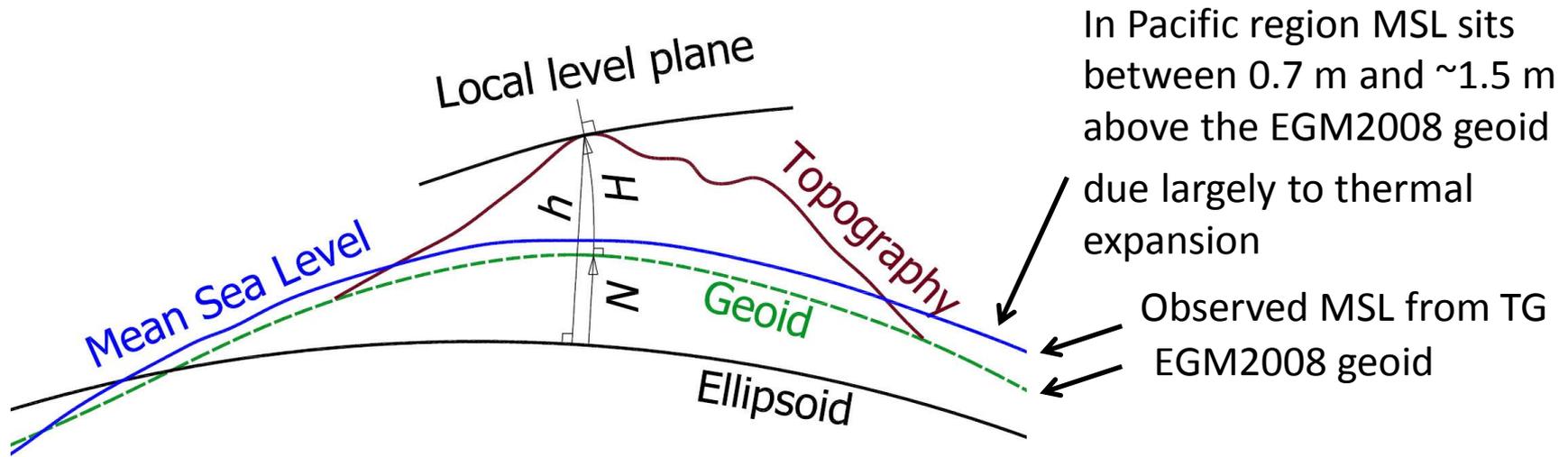
Gridded patch models of seismic deformation (including postseismic deformation)

Used in conjunction with secular (interseismic) deformation model.

If postseismic decay is significant, a gridded model of decay coefficients may be required



Develop local geoid model to fit observed MSL

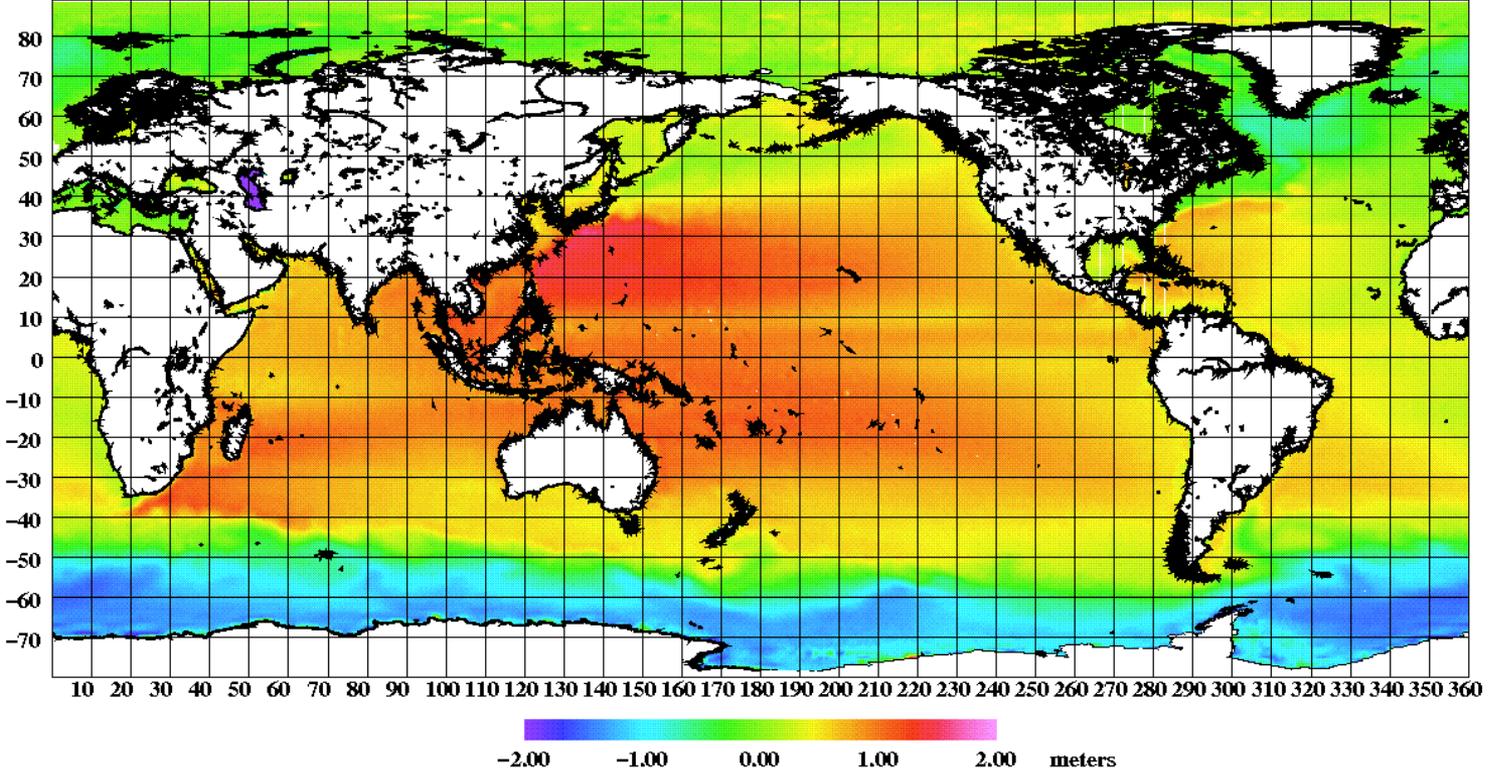


Offsets between observed MSL and EGM2008 can be interpolated (e.g. by kriging) and an “MSLoid” or hybrid geoid computed by adding offsets to EGM2008 N values

Other technique – using model of Mean Dynamic Topography (MDT) (ocean topography) from satellite altimetry

Difference between MSL and EGM2008

DNSC07MDT – Mean Dynamic Topography



Technical University of Denmark – National Space Institute

Geodetic Adjustment

ITRF2014 at mean epoch of measurement for fiducial network and GNSS baseline processing

Eliminate float or high RMS GNSS baselines
Evaluate weighting of fiducial station coordinates
Older baselines and legacy measurements not recommended

Loop closure - robustly isolate incorrectly weighted baselines

Run adjustment – tweak a priori and weighting to achieve RV of close to 1



Develop Map Grid(s) related to datum and ellipsoid

- UTM typically has large scale factors due to 6 deg wide zone
- UTM often not suitable for cadastral mapping and engineering surveys
- Options for best fitting map grid projection to keep scale factors close to 1.00000
- Selecting projection surface to coincide with mean elevation of region
- Local Transverse Mercator (LTM) (good for most jurisdictions) – Projection can be designed so that LTM bearings are aligned with underlying UTM grid bearings
- Stereographic Projection – Good for large square / circular regions
- Lamberts Conformal Conic – Good for higher latitude E-W shaped regions

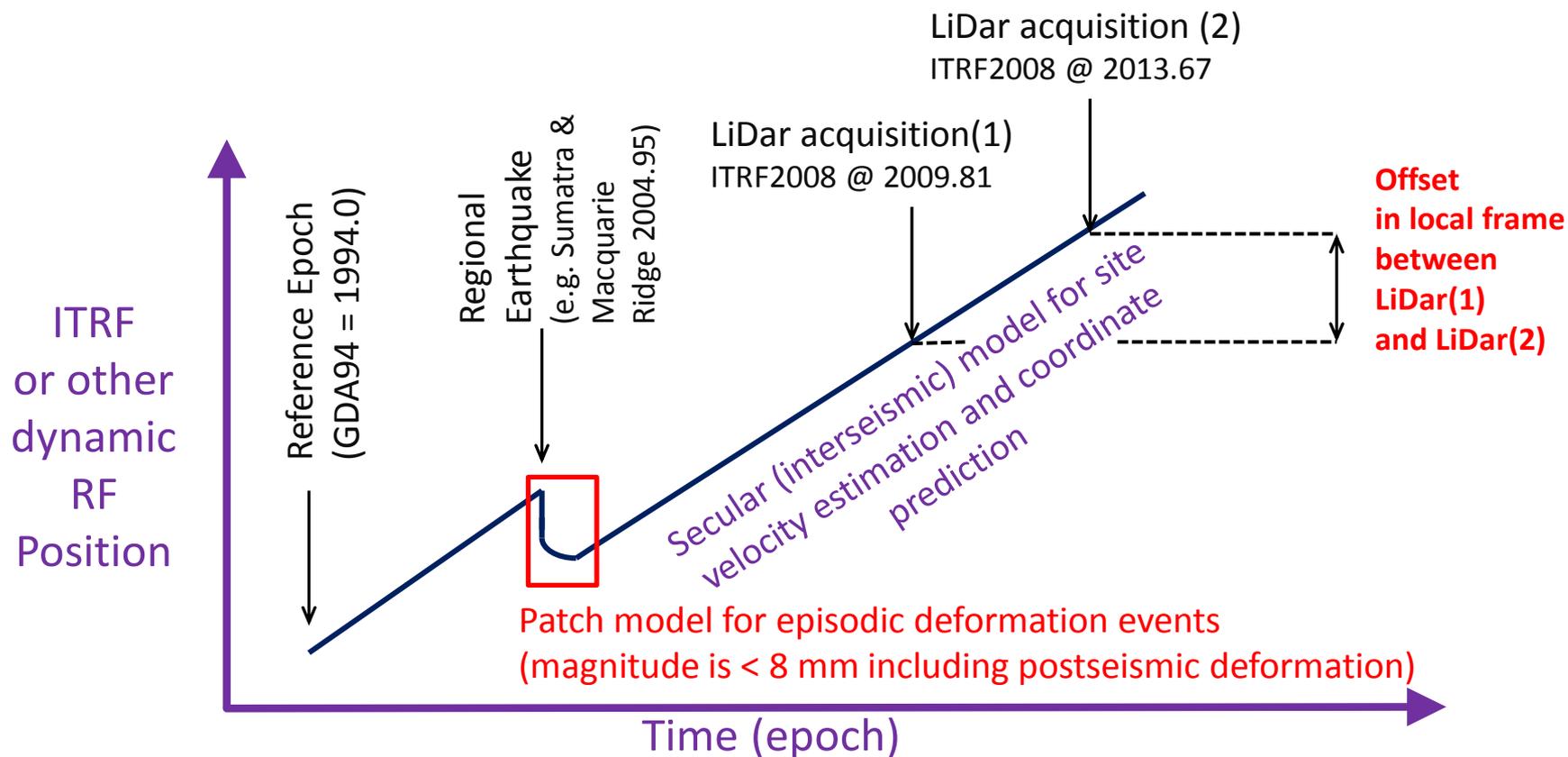


Compute transformation parameters from old datums

- Least squares estimation of transformation parameters by analysis of new datum and old datum coordinates.
- Requires robust filtering strategy (e.g. L1 Norm & eyeball) to isolate “rogue” coordinates and undocumented adjustment and realisation differences.
- 7-parameter model is the standard approach (mindful of larger rotations for reversibility), but also 3-parameter (small data sets) and distortion grids (e.g. NTV2)
- Need to provide parameters to GIS developers (e.g. ESRI, QGIS and MapInfo) EPSG, ISO-TC/211 and other geodetic registries

Kinematic ITRF and spatial data management

– care is required!





Publication of datum definition, coordinates, station summaries etc.

- Publish datum technical specifications on the web
- Station maps, coordinate lists, uncertainties / VCV - SINEX and station diagrams on web (preferably free access)
- Online portal for Rinex data from CORS
- Post-processing / RTK services
- Coordinate, elevation and datum conversion services
- Subscription access to RT data streaming (e.g. RTK, NTRIP)
- New Zealand has a particularly good model for dissemination of geodetic data to users



Example of datum access (New Zealand)

Web-page for data

[PositionNZ-RT – real-time data streams](#)
 Find out how to access real-time data from PositionNZ stations. [more...](#)
[Text only version](#)

Clickable map

Active Control Stations

Chatham Islands

Note: Map not to scale

PositionNZ stations Velocity Model Download data
 Details Timeseries Availability Photo

Data Access

Details for KAIK

[Info] [Antenna] [Photo] [Notes]

Code: [KAIK](#)

Name: Kaikoura GPS

DOMES number: 50231M001

Datum: New Zealand Geodetic Datum 2000

Latitude: 42° 25' 31.68098" S

Longitude: 173° 32' 1.17168" E

Ellipsoidal height: 314.809 m

Mark description: 5/8 thread in centre of 120 mm diameter stainless steel plate set in concrete pillar.

Receiver: TRIMBLE NETR9
 L1/L2+L2C/L5 GLONASS L1/L2 with 2 Maxwell-6 ASIC,

Antenna: [TRM55971.00](#)
 Zephyr GNSS Geodetic II - lead-based solder L1/L2/L5/G1/G2/G3/E1/E2/E5ab/E6/Compass

Antenna height: 0.0550

Coordinates and elevation
 (including historical)
 Uncertainty / class / order

Location Diagram

Mark and Site Details Form

Control Code: 4198 Mark Type: Boreas Pillar
 Name: 4023 Lead District: Marlborough
 Issue: P31A NZMG Ref: 76921
 Date of Inspection: 2003.01.21
 UTM Zone: 50 Date Established: 1985.01.21
 ID: 100 ID Place: 6405
 Structure Type: Post & Rail Enclosure
 Date Description: Post and rail enclosure surrounding mark

Access Diagram:
 11 m (approx. 36ft) north of Boreas Street, 100 m east of...

Physical Address:
 11 m (approx. 36ft) north of Boreas Street, 100 m east of...

Post Office:
 000 Telecom

Antenna Height:
 0.0550 m

Antenna Description:
 Zephyr GNSS Geodetic II - lead-based solder L1/L2/L5/G1/G2/G3/E1/E2/E5ab/E6/Compass

Antenna Reference Point:
 0.0550 m

Antenna info

Details for KAIK

[Info] [Antenna] [Photo] [Notes]

Code: [KAIK](#)

Receiver: TRIMBLE NETR9
 L1/L2+L2C/L5 GLONASS L1/L2 with 2 Maxwell-6 ASIC,

Antenna: TRM55971.00
 Zephyr GNSS Geodetic II - lead-based solder L1/L2/L5/G1/G2/G3/E1/E2/E5ab/E6/Compass

Antenna height: 0.0550

ARP: Antenna Reference Point
 TCR: Top of Choking
 TOP: Top of Ground Plane
 TDG: Top of Dome Ground Plane
 TPA: Top of Preamplifier
 TOP: Top of Pole

BCR: Bottom of Choking
 BOP: Bottom of Ground Plane
 BDG: Bottom of Dome Ground Plane
 BPA: Bottom of Preamplifier
 BMM: Bottom of antenna mount

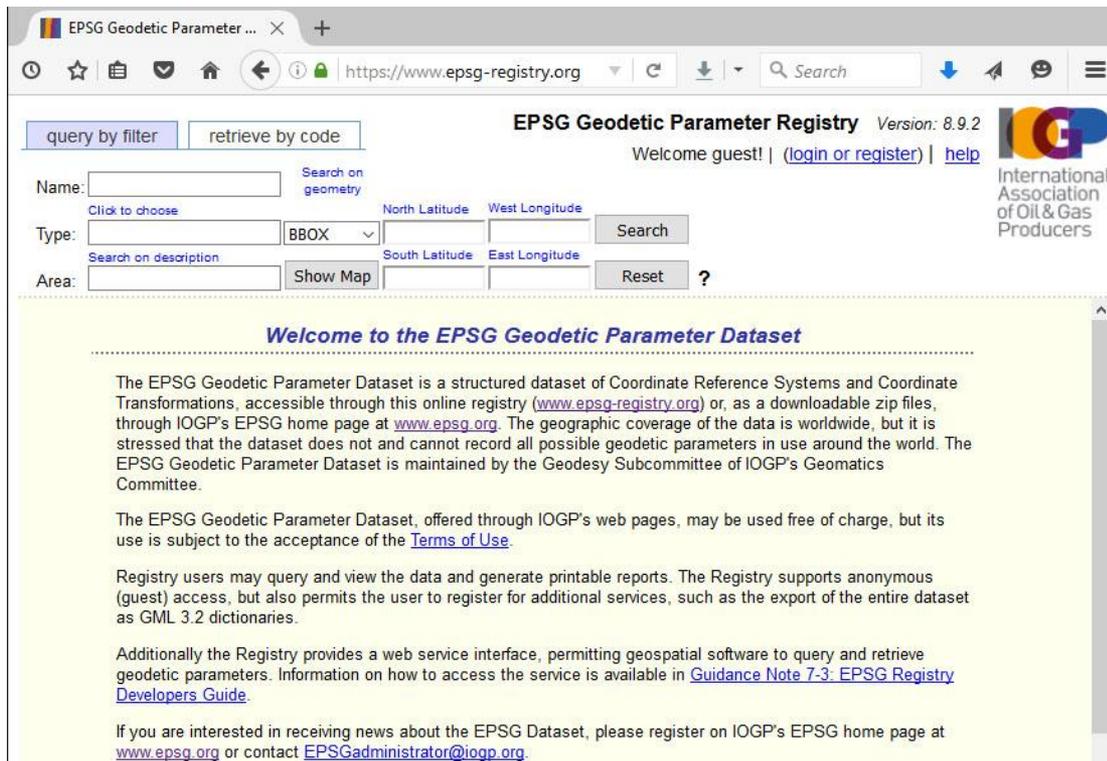
Details for KAIK



Station and mark photos

Submission to IOGP/EPSG and ISO TC/211

- EPSG and ISO TC/211 geodetic registries used as authoritative data for GIS updates.



The screenshot shows the EPSG Geodetic Parameter Registry website. The browser address bar displays <https://www.epsg-registry.org>. The page title is "EPSG Geodetic Parameter Registry" with a version number of 8.9.2. The page includes a search interface with fields for Name, Type, and Area, and buttons for "Search" and "Reset". The main content area is titled "Welcome to the EPSG Geodetic Parameter Dataset" and contains the following text:

The EPSG Geodetic Parameter Dataset is a structured dataset of Coordinate Reference Systems and Coordinate Transformations, accessible through this online registry (www.epsg-registry.org) or, as a downloadable zip files, through IOGP's EPSG home page at www.epsg.org. The geographic coverage of the data is worldwide, but it is stressed that the dataset does not and cannot record all possible geodetic parameters in use around the world. The EPSG Geodetic Parameter Dataset is maintained by the Geodesy Subcommittee of IOGP's Geomatics Committee.

The EPSG Geodetic Parameter Dataset, offered through IOGP's web pages, may be used free of charge, but its use is subject to the acceptance of the [Terms of Use](#).

Registry users may query and view the data and generate printable reports. The Registry supports anonymous (guest) access, but also permits the user to register for additional services, such as the export of the entire dataset as GML 3.2 dictionaries.

Additionally the Registry provides a web service interface, permitting geospatial software to query and retrieve geodetic parameters. Information on how to access the service is available in [Guidance Note 7-3: EPSG Registry Developers Guide](#).

If you are interested in receiving news about the EPSG Dataset, please register on IOGP's EPSG home page at www.epsg.org or contact EPGAdministrator@iogp.org.



GIS readable formats

OGC WKT (Human Readable)

```
PROJCS["NZGD2000 / New Zealand Transverse Mercator 2000",
  GEOGCS["NZGD2000",
    DATUM["New_Zealand_Geodetic_Datum_2000",
      SPHEROID["GRS_1980",6378137,298.257222101,
        AUTHORITY["EPSG","7019"]],
      TOWGS84[0,0,0,0,0,0,0],
      AUTHORITY["EPSG","6167"]],
    PRIMEM["Greenwich",0,
      AUTHORITY["EPSG","8901"]],
    UNIT["degree",0.01745329251994328,
      AUTHORITY["EPSG","9122"]],
    AUTHORITY["EPSG","4167"]],
  UNIT["metre",1,
    AUTHORITY["EPSG","9001"]],
  PROJECTION["Transverse_Mercator"],
  PARAMETER["latitude_of_origin",0],
  PARAMETER["central_meridian",173],
  PARAMETER["scale_factor",0.9996],
  PARAMETER["false_easting",1600000],
  PARAMETER["false_northing",10000000],
  AUTHORITY["EPSG","2193"],
  AXIS["Easting",EAST],
  AXIS["Northing",NORTH]]
```

- No standard yet for time-dependant transformations in GIS

ESRI WKT

```
PROJCS["NZGD2000 / New Zealand Transverse Mercator
2000",GEOGCS["NZGD2000",DATUM["D_NZGD_2000",SPHEROID["GRS_19
80",6378137,298.257222101]],PRIMEM["Greenwich",0],UNIT["Degr
ee",0.017453292519943295]],PROJECTION["Transverse_Mercator"]
,PARAMETER["latitude_of_origin",0],PARAMETER["central_meridi
an",173],PARAMETER["scale_factor",0.9996],PARAMETER["false_e
asting",1600000],PARAMETER["false_northing",10000000],UNIT["
Meter",1]]
```

PROJ4

```
+proj=tmerc +lat_0=0 +lon_0=173 +k=0.9996 +x_0=1600000
+y_0=10000000 +ellps=GRS80 +towgs84=0,0,0,0,0,0,0 +units=m
+no_defs
```



Thank You!

Richard Stanaway

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or

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This presentation will soon be available:

<http://www.quickclose.com.au/rfip2016.pdf>