Efficient and Well-documented Digital Orthophoto Production from Airborne Photogrammetry

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Key words: Digital Terrain Models, 3D city models, conventional orthophotos, true orthophotos, orthophoto quality measures.

SUMMARY

Digital orthophotos offer a valuable level of detail in their coexistence with accurate vector data. Large and medium scale airborne photogrammetry typically leads to orthophoto ground sample distances in the range 0.10 - 0.40 meter, thus offering a better level of detail than the commercial satellite products that have entered the marketplace over the last few years.

The usage of orthophotos together with 3D city models is becoming popular in conjunction with modern urban mapping. With all buildings in the 3D model registered in their exact position with both planar and height coordinates, the generation of orthophotos with vertical buildings and hidden areas filled in from adjoining images becomes possible. The generation of such geometrically correct ("TRUE") orthophotos differs significantly from conventional orthophoto projects with stricter requirements to the digital terrain -and building models and with the need for sophisticated visibility analysis implemented in the orthophoto routine. BlomInfo A/S and PT. Blom Nusantara use this methodology to describe urban topography in very precise, cost-efficient and highly illustrative manners.

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1. INTRODUCTION

Orthophoto production concerns the process of transforming the central projection of the terrain as recorded in aerial photos into a digital geo-referenced image (orthophoto) with the same geometric attributes as a map. This transformation from a central projection into an orthogonal projection requires a Digital Terrain Model (DTM) with sufficient density and accuracy.

The complexity of digital orthophoto production depends on a number of different project characteristics and varies from fairly straightforward processes to more complex and time-consuming workflows. The required positional accuracy and the resolution of the orthophoto pixels on the ground are two of the most dominant orthophoto project characteristics. Orthophoto projects are usually delivered to the end user as a seamless mosaic of orthophoto map-sheets in which all seam-lines (edges) between the individual orthophotos have been made invisible.

If building structures are to be placed at their exact geometric position in the orthophotos, this requires all 3D structures to be included in the DTM and to be carefully analyzed and processed by the orthophoto generation software. Normally, orthophotos produced from a DTM on the ground are referred to as orthophotos and the more advanced version with geometrically correct building structures are referred to as "TRUE" orthophotos.

2. ORTHOPHOTO PROJECT BREAKDOWN

Orthophoto projects have a lot in common with any other airborne photogrammetric mapping project. The flight-mission, recording of digital source image material and procedures for establishing accurate Exterior Orientation are therefore not described in this paper. However, it is underlined that these procedures will have a direct impact on the geometric accuracy of the produced orthophoto.

After an acceptable Exterior Orientation has been established, a typical orthophoto project can be divided into the following chronological procedures:





3. DTM IMPLEMENTATION AND ORTHOPHOTO ACCURACY

The geometric accuracy of any produced orthophoto is directly linked to the quality of the DTM. Inaccurate DTM data is the most dominant source of error in the produced orthophotos and the workload required to establish a satisfactory DTM is, of course, very dependent on the topography in the project area. As stated in the previous section, the DTM is usually derived from one or a combination of the following methods:

- Manual stereo registrations, usually in a combination of grid and breaklines.
- Automatic measurements (correlation).
- Contour lines from older topographic maps (usually supported by other terrain features).
- Laser scanning.

Objects elevated a distance (dH) above the DTM will be pictured a radial distance (dR) outwards from the center of the image as given by the formula:

 $dR = (r/c) \cdot dH$

where

r is the distance from the center of the image and c is the focal length.

The same formula can be used to calculate Fig. 2: Radial displacement (dR) for an object positional errors (dR) in the orthophoto caused by errors in the DTM (dH).



elevated above the terrain surface.

Different software offers many different methods and routines for dynamic filtering, quality improvement and accuracy documentation of the DTM. One way to document the DTM accuracy is to measure a statistically sufficient number of random points on the terrain in the photogrammetric stereo models and compare these height-coordinates with corresponding interpolated height values in the DTM. Statistics can then be calculated and documented as suggested in figure 3 a. and 3 b. Note that as long as both the DTM registrations and the control measurements are made from the same imagery with the same Exterior Orientation, these control measurements do not express the final accuracy of the DTM in the terrain.

A	В	С	D	E	F	G
Point No.	E	N	Н	H interpolated in DTM	Delta H	
1	492775.843	4717484.761	599.232	599.695	0.463	
2	492757.439	4717624.263	599.191	599.683	0.492	
3	492756.903	4717752.890	596.340	597.697	1.357	
210	491471.486	4722248.638	625.505	623.790	-1.715	5 K
211	490961.028	4722288.677	633.568	630.697	-2.871	36
212	491561.498	4722537.336	640.538	640.798	0.260	
213	492493.213	4722468.818	603.038	601.512	-1.526	
214	491289.246	4722078.837	616.518	617.447	0.929	
214 Points				RMSE	0.859	
1						
				Average	-0.222	
				Standard Dev	0.831	
				Largest Dev	-3.047	
				Specified gross error	4.20	(from specification)
				No. of gross errors	0	
	A Point No. 1 2 3 210 211 212 213 214 214 Points	A B Point No. E 1 492775.843 2 492757.439 3 492756.903 210 491471.486 211 490961.028 212 491561.498 213 492493.213 214 491289.246 214 Points	A B C Point No. E N 1 492775.843 4717484.761 2 492757.439 4717624.263 3 492756.903 4717752.890 210 491471.486 4722248.638 211 490961.028 4722288.677 212 491561.498 4722537.336 213 492493.213 4722468.818 214 491289.246 4722078.837 214 Points Image: Content of the state of the sta	A B C D Point No. E N H 1 492775.843 4717484.761 599.232 2 492757.439 4717624.263 599.191 3 492756.903 4717752.890 596.340 210 491471.486 4722248.638 625.505 211 490961.028 4722288.677 633.568 212 491561.498 4722537.336 640.538 213 492493.213 4722468.818 603.038 214 491289.246 4722078.837 616.518 214 Points 213 492493.213 4722078.837 616.518	A B C D E Point No. E N H H interpolated in DTM 1 492775.843 4717484.761 599.232 599.695 2 492757.439 4717624.263 599.191 599.683 3 492756.903 4717752.890 596.340 597.697 210 491471.466 4722248.638 625.505 623.790 211 490961.028 4722288.677 633.568 630.697 212 491561.498 4722537.336 640.538 640.798 213 492493.213 4722488.818 603.038 601.512 214 491289.246 4722078.837 616.518 617.447 Z14 Points RMSE RMSE Image: Context Standard Dev Largest Dev Largest Dev Image: Context Standard Dev Largest Dev No. of gross error	A B C D E F Point No. E N H H interpolated in DTM Delta H 1 492775.843 4717484.761 599.232 599.695 0.463 2 492757.439 4717624.263 599.191 599.683 0.492 3 492756.903 4717752.890 596.340 597.697 1.357 210 491471.466 4722286.677 633.568 630.697 -2.871 211 490961.028 4722537.336 640.538 640.538 601.512 -1.526 213 492493.213 4722488.818 603.038 601.512 -1.526 214 491289.246 4722078.837 616.518 617.447 0.929 214 Points P P P P P P 214 491289.246 472078.837 616.518 617.447 0.929 P 214 Points P P P P P P P P

Fig. 3 a: Reported statistics for height differences between control measurements made on the ground in the photogrammetric stereo models and corresponding height values interpolated in the DTM. (Example from project photographed in 1:12000 image scale with normal-angle camera, c =0,303 m).



Fig. 3 b: Graphical illustration of the 214 control measurements in figure 3 a. The average height difference of minus 0,222 meter ("H interpolated in the DTM" minus "H measured") means that on average this DTM lies 0,222 meter below the 214 control measurements.

4. CONVENTIONAL ORTHOPHOTOS

In combination with accurate vector data, orthophotos facilitate the interpretation and analysis of the map data. Color orthophotos are typically preferred to grayscale orthophotos as colors help increase these advantages. However, grayscale orthophotos will often be a good option on projects where budget limitations and other project parameters suggest a grayscale approach.



Fig. 4 a: Only vector data ("top" view).



Fig. 4 b: 24-bit color orthophoto, 0.10 m GSD.



Fig. 4 c: Combined 24-bit color orthophoto pixels and vector data ("top" view). (Danish TK3 / TK99 mapping standard).



Fig. 5: 8-bit grayscale orthophoto, 0.20 m GSD. (Norwegian SOSI mapping standard).

Large and medium scale airborne photogrammetry typically leads to orthophoto ground sample distances (GSD) in the range 0.10 - 0.40 meter and most orthophoto production lines handles grayscale, color and infrared imagery.



Fig. 6: 24-bit color orthophoto, 0.10 m GSD.Fig. 7: Infrared orthophoto, 0.50 m GSD.

Because conventional orthophotos are generated from DTMs representing the terrain and do not include buildings and other features above this DTM surface, such 3D objects are not pictured at their exact geometric position. Thus, the geometric accuracy of conventional orthophotos must be checked up against features on the ground. Because the radial distortion increases with the distance to the image center, orthophotos are normally generated from the center of each aerial source image. Note that this differs from the photogrammetric stereo registration where the stereo overlap between the aerial photos must be the area unit.

5. 3D CITY MODELLING AND TRUE ORTHOPHOTOS

3D city models have become popular in conjunction with modern urban mapping. These digital terrain models very precisely describe urban topography and can, for example, be used for urban planning, telecom applications, environmental analysis, risk management, transportation logistics and visualization of proposed developments.



Fig. 8 a: 3D City Model, Copenhagen. (BlomInfo A/S - Denmark).

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Fig. 8 b: Screen-dump from a 7 minutes fly through simulation in Copenhagen. (BlomInfo A/S - Denmark).



Fig. 8 c: Screen-dump from a 5 minutes fly through simulation in Copenhagen. (BlomInfo A/S - Denmark).

Generation of geometrically correct "TRUE" orthophotos with vertical buildings and hidden areas filled in from adjoining images can be divided into two major work tasks. First, all 3D objects have to be included in the DTM. This operation usually combines a Digital Terrain Model (DTM) on the ground with a Digital Building Model (DBM) to form a Digital Surface Model (DSM). Principally, this DSM can also be a product from other methods such as very dense automatic correlated measurements or airborne laser scanning, but manual photogrammetric registrations currently seem to have an advantage in the way they represent the exact shape of the building structures.

Secondly, generation of geometrically correct "TRUE" orthophotos requires sophisticated visibility analysis implemented in the orthophoto routine so that hidden areas can be filled in from other neighboring images. Figure 9 a. and 9 b. illustrate different relief displacements

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and different hidden areas due to different perspective center positions. In the visibility analysis, the hidden areas are identified and orthophoto pixels for these areas are generated and inserted from adjoining source imagery. Color balancing and seam-line feathering are important routines in these processes.



Fig. 9 a: Hidden areas from different perspective centers. (Morten Nielsen).



Fig. 10 a: Conventional orthophoto from DTM on the ground ("top" view).



Fig. 9 b: Common area (yellow) from 4 different perspective centers. 60% image overlap both forward and sideways. (Morten Nielsen).



Fig. 10 b: Building structure included in DTM/DBM to form a DSM ("isometric" view).



Fig. 10 c: Orthophoto with correct vertical roof and indicated hidden area (in blue) to be filled from adjoining images ("top" view). Note that this screendump was made <u>before</u> ortho-pixels were automatically generated and inserted from an adjoining image.

Figure 10 a. -10 c. illustrate the difference between a conventional orthophoto from a DTM on the ground and a "TRUE" orthophoto <u>before</u> orthophoto pixels are automatically generated and inserted into the calculated hidden area from a suitable neighboring image.

Figure 12 a. and 12 b. show calculated hidden areas for the building shown to the right (figure 11.). As seen in figure 12 a., this building is visible in a total of 4 aerial photos, namely image 0103 and 0104 in the Northern strip and image 0203 and 0204 in the Southern strip.

Because the relief displacements will be much less significant for buildings located closer to any perspective center, most "TRUE" orthophoto projects will be flown with larger sideways overlap than the 20% seen in figure 12 a.



Fig. 11: Building structure included in the DTM ("isometric" view).

Relief displacements can also be significantly reduced by using a normal-angle camera instead of a wide-angle camera, but with the need of either flying twice as high to cover the same area as with a wide-angle camera or adding more aerial photos to the project.



Fig. 12 a: Adjoining orthophoto generation shapes in two neighboring strips.



Fig. 12 b: Calculated hidden areas from uSMART softcopy. 60% forward and 20% sideways overlap. The calculated hidden areas must be filled with orthophoto pixels from the most suitable neighboring source image. Depending on the location of the building structure, this can be a source image in the same strip, in a neighboring strip or possibly in a cross-strip previously

used for minimizing the ground control in the Aerial Triangulation component of the photogrammetric mapping project.



Fig. 13: Conventional orthophoto (left) and "TRUE" orthophoto (right), both overlaid with the same vector map of the building outlines. (Morten Nielsen).

Figure 13. illustrates the significant advantage of "TRUE" orthophotos in built up areas. The conventional orthophoto has a good fit with objects in level with the terrain, but not with the building structures that were not included in the DTM used for the ortho-rectification. This effect has been eliminated in the "TRUE" orthophoto.

6. SUMMARIZING REMARKS

It is expected that there will be a continuous and increasing demand for both conventional and "TRUE" orthophotos. What will be the most cost-efficient option of these two products will vary depending on different project characteristics and end-user demands.

If there is a requirement that all 3D building structures are to be placed at their exact geometric position, the "TRUE" orthophoto workflow requires that the orthophotos must be generated in more complex manners with the introduction of more time consuming processes in both the establishment of the DTM and in the orthophoto rectification process.

Both conventional orthophoto production and "TRUE" orthophoto production are highly automated processes and the cost difference between the two products is highly related to the level of automatization. A current and great challenge is to make the "TRUE" orthophoto production as automatic as possible and thereby minimize the cost difference between these two products.

The establishment of a Digital Building Model (DBM) and a Digital Surface Model (DSM) that include all 3D structures in the project area is by far the most labor-intensive part of a "TRUE" orthophoto project. If these models already exist, this significantly lowers the threshold for a "TRUE" orthophoto approach.

If the orthophoto is to be draped over a 3D city model, only "TRUE" orthophotos will have a perfect fit onto the 3D building model.

For both production lines, it is important that automatic procedures are closely combined and integrated with manual quality control and satisfactory documentation routines.

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BIOGRAPHICAL NOTES

Hakon Andresen graduated from the Department of Mapping Sciences at The Norwegian Agricultural University in 1999. His experience includes studies at the University of Calgary (Canada), two years with Blom in Abu Dhabi (United Arab Emirates) and he is at present Production Manager in PT. Blom Nusantara in Bandung (Indonesia).

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