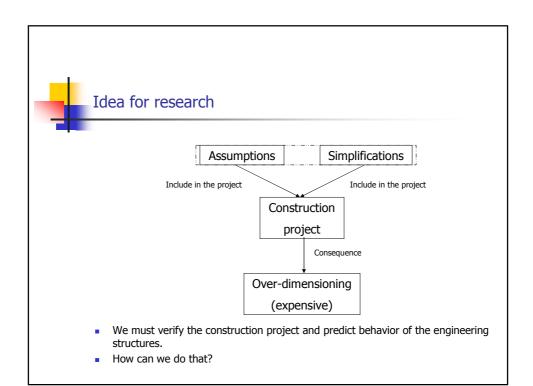
# Modeling of Engineering Structures Displacement by using the Euler Method

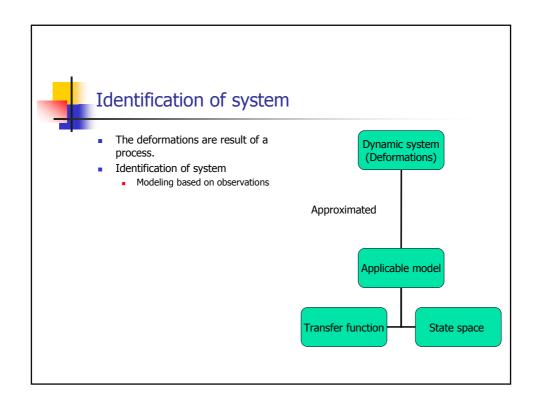


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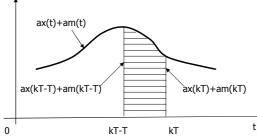


#### The Euler Method

 It is a first order numerical procedure for solving ordinary differential equations with a given initial value.

$$G(s) = \frac{X(s)}{M(s)} = \frac{a}{s+a}$$
 - transfer function of the first order function

$$x(kT) = (1 - aT)x(kT - T) + Tam(kT - T)$$





## Subspace method

A linear system can be represent in state space form:

$$\mathbf{x}(kT + T) = \mathbf{A}\mathbf{x}(kT) + \mathbf{B}\mathbf{u}(kT) + \mathbf{w}(t)$$

$$\mathbf{y}(kT) = \mathbf{C}\mathbf{x}(kT) + \mathbf{D}\mathbf{u}(kT) + \mathbf{v}(kT)$$

where:

 $\mathbf{x}$  is a n-dimensional state vector

 $\mathbf{u}$  is a  $n_{u}$  -dimensional input vector

y is a  $n_y$  – dimensional output vector

 ${\bf v}$  is a  $n_v$  –dimensional noise vector

 $_{\mathbf{w}}$  is a  $_{n}$ -dimensional process noise vector

A, B, C and D are parameter matrices of the appropriate dimension

$$\mathbf{Y}(kT) = \begin{bmatrix} \mathbf{x}(kT+T) \\ \mathbf{y}(kT) \end{bmatrix}, \quad \Theta = \begin{bmatrix} \mathbf{A} & \mathbf{B} \\ \mathbf{C} & \mathbf{D} \end{bmatrix}$$
$$\mathbf{\Phi}(kT) = \begin{bmatrix} \mathbf{x}(kT) \\ \mathbf{u}(kT) \end{bmatrix}, \quad \mathbf{E}(kT) = \begin{bmatrix} \mathbf{w}(kT) \\ \mathbf{v}(kT) \end{bmatrix}$$

$$\mathbf{Y}(kT) = \mathbf{\Theta}\mathbf{\Phi}(kT) + \mathbf{E}(kT)$$



## Structures displacement

Equation of motion / main equation in Civil Engineering practice

$$M\ddot{x}+C\dot{x}+Kx=F$$

where:

M is the mass,

K is the stiffness,

C is damping coefficient,

x is displacment response of the structure to some extrenal force F, where the dots above the displacment variable are indicative of the derivation order

Euler method

The model is translated into the state-space differential equations.

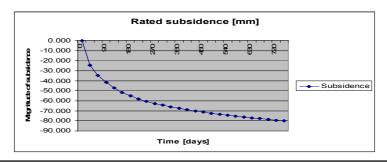
$$\begin{split} \dot{x}_1(t) &= x_2(t) \\ \dot{x}_2(t) &= -\frac{C}{M} x_2(t) - \frac{K}{M} x_1(t) + \frac{F}{M} = -a x_2(t) - b x_1(t) + m \\ & \left[ x_1 \left[ (k+1)T \right] \right] = \begin{bmatrix} 1 & T \\ -bT & 1 - aT \end{bmatrix} \begin{bmatrix} x_1(kT) \\ x_2(kT) \end{bmatrix} + \begin{bmatrix} T \\ 0 \end{bmatrix} m \,, \end{split}$$

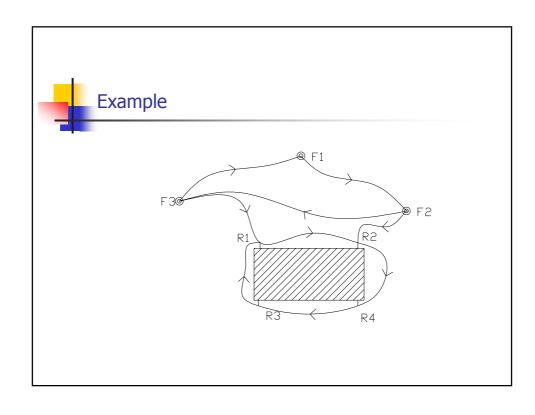


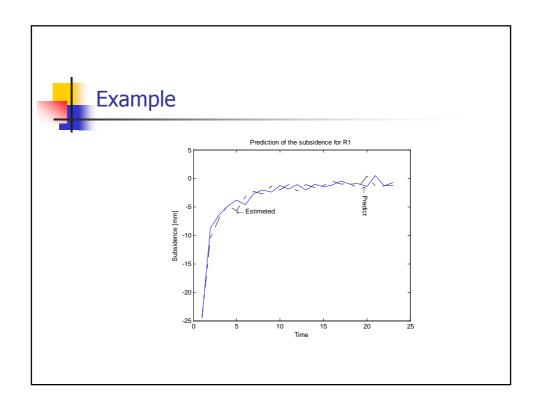
## Example

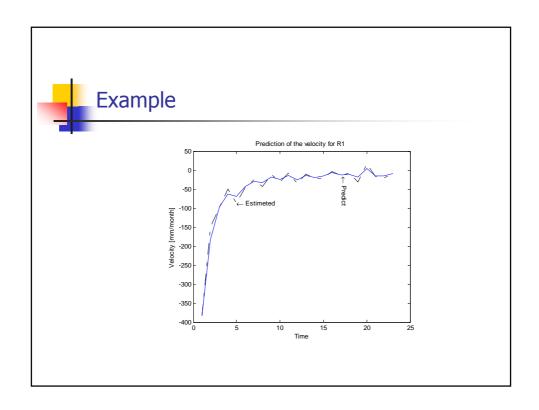
A wide area is loaded with 0.1 MN/m² for a short period of time. The terrain profile is a mediaeval plastic clay, with the thickness of 5.5 m, horizontally intersected by a sand layer of 0.5 m thickness. Underneath the clay layer is an incompressible and impermeable substrate. The clay's module of compressibility is 5.0 MN/m². The coefficient of consolidation is  $5*10^{-4}$  cm²/s.

- 26 epoch of observation simulated
  - o 0.5 mm (per station)
  - o T=0.0822 monthly











#### Conclusion

- Deformation process non-linear
- We can use successfully linear method subspace method for subsidence approximation
- System's ill-conditioning
- For prediction is enough only five epoch of measurements

Same results I got when I use transfer function models

- First order dynamical system- three epoch need for prediction
- AR model where the order of model is decided by autocorrelation function - need four epoch

